

Experimental Study on Flexural Behavior of Bendable Concrete

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Abstract— Bendable Concrete also known as Engineered Cementitious Composites abbreviated as ECC is class of ultra-ductile fiber reinforced cementitious composites, characterized by high ductility and tight crack width control. This material is capable to exhibit considerably enhanced flexibility. An ECC has a strain capacity of more than 3 percent and thus acts more like a ductile metal rather than like a brittle glass. A bendable concrete is reinforced with micromechanically designed polymer fibres. The aim of this study is to investigate the hardened property (i.e. Flexural Test) of ECC by addition of AR-Glass fibres in different proportion. The result is a moderately low fiber volume fraction (<2%) composite which shows extensive strain-hardening.

Key words: *Bendable Concrete, ECC-Engineered Cementitious Composites, Deflection,*

1. Introduction:

Engineered Cementitious Composites Concrete (ECC) also called as Bendable Concrete or Conflexpave, is an easily moulded mortar-based composite reinforced with specially selected short random fibers usually polymer fibers. ECC acts more like a ductile metal than a brittle glass which then leads to a wide variety of application. The tensile strain capacity of ECC can reach 3-5%, compared to 0.01% for normal concrete. The compressive strength of ECC is similar to that of normal to high strength concrete. The aim of research work is to study ductile behavior of concrete, crack resistance capacity & concrete should give warning before its failure. Normal concrete is brittle in nature while ECC is ductile in nature, due to this property; it has wide applications & wide future scope in various fields.

1.1 OBJECTIVES OF THE INVESTIGATION:

- To check the behaviour of ECC-bendable concrete under compression, Split Tensile Test & Flexure Test.
- To find the deflection of ECC beams.

2. INGREDIENTS OF ECC CONCRETE:

Engineered cementitious composite is composed of cement, sand, fly ash, water, small amount of admixtures and an optimal amount of fibers. In the mix coarse aggregates are deliberately not used because property of ECC Concrete is formation of micro cracks with large deflection. Coarse aggregates increases crack width which is contradictory to the property of ECC Concrete.

2.1 CEMENT

The cement is called Portland slag cement (PSC) because of that Blast- furnace slag may also be used in some cements and The color of the cement is due chiefly to iron oxide. In the absence of impurities, the color would be white, but neither the color nor the specific gravity is a test of quality. Ordinary Portland cement 53 grade (Ultratech Cement) was use, the specific gravity is 3.15.

2.2 SAND [FINEAGGREGATE]

Fine aggregate / natural sand is an accumulation of grains of mineral matter derived from the disintegration of rocks. It is distinguished from gravel only by the size of the grains or particles, but is distinct from clays which contain organic materials. The most useful commercially are silica sands, often above 98% pure. Sand is used for making mortar and concrete and for polishing and sandblasting. Sands containing a little clay are used for making molds in foundries. The sand passed through of 4.75 mm sieve is used which was available locally. The specific gravity of sand is 2.60 and water absorption rate of 1.23%.

2.3 SUPERPLASTISIZER

VARAPLAST PC 432 is a ready to use admixture that is added to the concrete at the time of batching. The maximum effect is achieved when the VARAPLAST PC 432 is added after the addition of 50% -70% of the water. VARAPLAST PC 432 must not be added to the dry material. Thoroughly mixing is essential and a minimum mixing cycle, after the addition of VARAPLAST PC 432 of 60 seconds for forced action mixers is recommended. The properties of Varaplast PC 432 are shown in Table 1.

Table 1: Properties of Varaplast PC 432

S.NO	Characteristics	Proportion
1	Calcium chloride Content	Nil
2	Specific gravity	1.08 at 30°C
3	Air entrainment	Less than 1% additional air is entrained.
4	Setting time	1-4 hours retardation depending on dosage and climate conditions.
5	Chloride content	Nil to BS5075
6	Colour	Brown

2.4 FLY ASH

Fly ash used was pozzolone class F. And specifications provided in Table 1 below. In RCC construction use of fly ash has been successful in reducing heat generation without loss of strength, increasing ultimate strength beyond 180 days, and providing additional fines for compaction. Replacement levels of primary class fly ash have ranged from 30-75% by solid volume of cementitious material. Class F fly ash is utilized so the acquisition cost is reduced. Only transportation cost is estimated. The properties of Fly ash are shown in Table 2.

S.NO	Characteristics	Proportion
1	ROS 45 microns sieve (max)	18
2	Loss on ignition	2.5
3	Water requirement	95%
4	Moisture content (max)	0.5
6	Lime reactivity (min)	5
7	SiO ₂ +Al ₂ O ₃ +Fe ₂ O ₃	90 min
8	SiO ₂	50 min
9	CaO	5 max
10	MgO	4 max
11	SO ₃	2 max
12	Na ₂ O	1.5 max

Table 2: Properties of Fly ash

2.5 AR Glass Fiber

AR Glass fibers also known as an alkali resistance glass fiber. Generally, glass consists of quartz, soda, sodium sulphate, potash, feldspar and a number of refining and dyeing additives. Glass fibers are useful because of their high ratio of surface area to weight. However, the increased surface area makes them much more susceptible to chemical attack. Humidity is an important factor in the tensile strength. The properties of AR Glass fibers are shown in Table 3.

Table 3: Properties of AR Glass fibers

S.NO	Characteristics	Proportion
1	Fibre	AR Glass
2	specific gravity	2.68
3	elastic modulus (Gpa)	72
4	tensile strength (Mpa)	1700
5	diameter (micron)	14
6	length (mm)	12
7	Aspect ratio	857.14

2.6 WATER

Water fit for drinking is generally considered fit for making concrete. Water should be free from acids, oils, alkalis, vegetables or other organic impurities. Soft waters also produce weaker concrete. Water has two functions in a concrete mix. Firstly, it reacts chemically with the cement to form a cement paste in which the inert aggregates are held in suspension until the cement paste has hardened.

3. ECC MIX DESIGN

Micromechanics are a branch of mechanics applied at the material constituent level that captures the mechanical interactions among the fiber, mortar matrix, and fiber-matrix interface. Typically, fibers are of the order of millimeters in length and tens of microns in diameter, and they may have a surface coating on the nanometer scale. Matrix heterogeneities in ECC, including defects, sand particles, cement grains, and mineral admixture particles, have size ranges from nano to millimeter scale.

3.1 PROPORTION OF ECC CONCRETE

Mass of Cement kg/m ³	Mass of fly ash kg/m ³	Mass of fine aggregate kg/m ³	Mass of water kg/m ³	Water Cement ratio
360	180	1080	144	0.40

3.2 Casting and Curing

Finally we have to choose the mix proportion was 1:0.5:2, AR Glass fiber 1%, 1.5%, 2% and superplasticizer dose was 500ml/bag and water to cementitious material ratio was 0.40. The casting was done according to IS:516 for mixing, mixed material was taken and filled into cubes, beams and cylinders for different testing. Specimens were taken out after 24 hours and put for curing for durations of 7, 14, 28 days.

3.3 Specimen Preparation

Add sand, cement, 50% of fly ash & 50% water & super plasticizer. Add slowly remaining quantity of fly ash, water & super plasticizer. Once the homogenous mixture is formed, add the AR glass fibers slowly. Mix all the constituents till the fibers are homogeneously mixed in the matrix. Additionally, slight adjustment in the amount of HRWR is performed to achieve consistent rheological properties for better fiber distribution and workability. The mixtures are then cast into molds and demolded after 24 hr. After being demolded, ECC specimens are cured in curing room where the temperature is 25°C for 7 and 28 days respectively.

4. RESULT AND DISCUSSION

4.1 SLUMP TEST

Slump test is used to determine the workability of fresh concrete. Due to a huge slump may obtain if there is any disturbance in the process. It also mentioned that a slump more than 225mm will indicate a very runny concrete. The apparatus & equipment used for the slump test & the procedure of the test according to IS7320-1974.

4.2 Compression test result

According to IS:509-1959, the testing for the specimens should be carried out as soon as possible after taking out from the curing tank. The specimen needs to get measurement before testing. The length and height of specimen is measured and recorded. The axis of specimen is aligned with the centre of thrust of the seated plate. Plate is lowered until the uniform bearing is obtained. The force is applied and increased continuously at a rate equivalent to 20MPa compressive stresses per minute until the

specimen failed. Record the maximum force from the testing machine. The observation from our results shows that the increase in compressive strength is up to 28% increase of adding 1%, 1.5% and 2% fibre content in comparison of conventional concrete. It shows that variation in compressive strength by adding fiber.



Fig. 1 Test on Cube

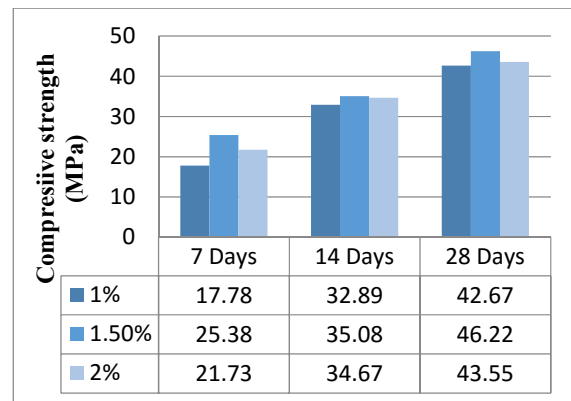


Fig.2 Compressive strength

4.3 Split tensile result

Normally concrete is very strong in compression but weak in tension. Indirect tensile test is used to indicate the brittle nature of specimens. Concrete is not elastic material. The stress strain behavior of concrete is straight line up to 10-15% of its ultimate strength. Split tensile test was performed on cylinder of size on universal testing machine according to IS: 5816-1999. The failure load to each cylinder was noted for finding split tensile strength.

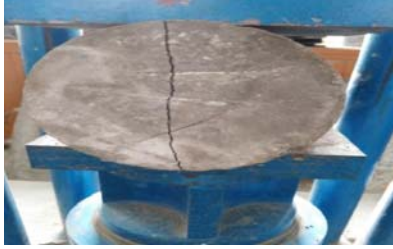


Fig.3 Test on cylinder

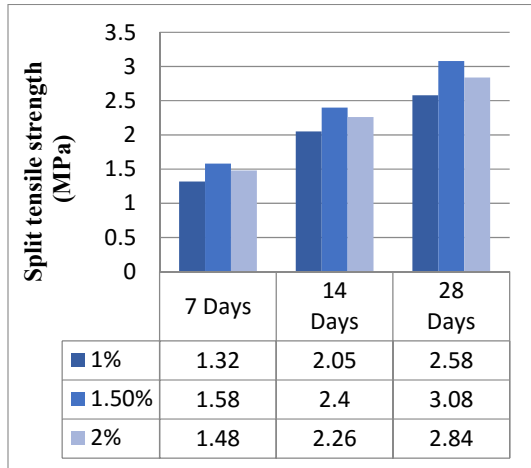


Fig.4 Split tensile strength

4.4 FLEXURAL TEST

The beam mould of size 100 X 15 X 10 cm (when size of aggregate is less than 40mm). The specimen shall be supported on 38mm diameter roller with 1000mm span for 150mm size specimen. The load shall be applied through two similar rollers mounted at the two points of the supporting span that is spaced at 40mm. The load is applied without shock at a rate of 4KN/minute for 150mm specimen. The load shall be increased until the specimen fails and the maximum load applied to the specimen during the test. The flexural test was performed on beams on universal testing machine according to IS: 516-1959. The failure load to each beam was noted for finding flexural strength. Table 4 shows the results on flexural testing.

Flexural strength of beam can be calculated by following formula,

$$f_b = PL / bd^2$$

Where,

- P = Maximum load in kN applied to the specimen
- L = length of the specimen in mm
- d = depth measured in cm of the specimen at the point of failure
- b = measured width of the specimen in mm

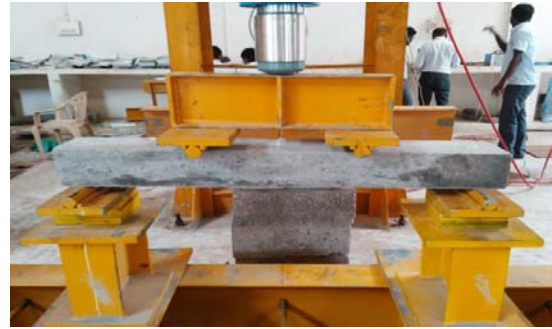


Fig. 5 Test on Beam

Table 4: Flexural strength result

Fiber content (%)	First Crack Strength on 14 days (MPa)		Ultimate Strength on 14 days (MPa)	
	Without reinforcement	With 6mm reinforcement	Without reinforcement	With 6mm reinforcement
1	6.1	10.6	7.8	11.7
1.5	6.5	10.25	7.3	11.9
2	6.3	10.50	7.8	11.7

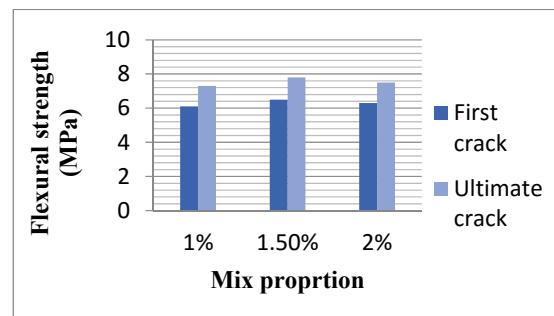


Fig. 6 Without reinforcement

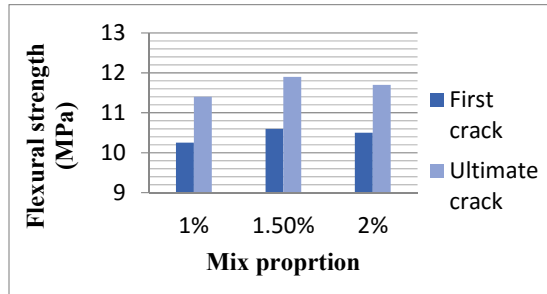


Fig. 7 With 6mm reinforcement

5. CONCLUSION

- ✓ According to test results, the beam is with standing high load and a large deformation without succumbing to the brittle fracture typical of normal concrete, even without the use of steel reinforcement.
- ✓ The significant properties of ECC Concrete are ductility, durability, compressive strength, and self-consolidation. Although the cost procured for the designing of ECC is normally higher than that of the normal concrete but it has numerous potential application.
- ✓ The percentage increase of compressive strength of various grades of AR glass fibre concrete mixes compared with 28 days compressive strength is observed 37%.
- ✓ The percentage increase of flexure strength of various grades of AR glass fibre concrete mixes compared with 28 days compressive strength is observed 5.19%.
- ✓ The result is a moderately low fiber volume fraction (<2%) composite which shows extensive strain-hardening, with strain capacity of about 3 to 5% compared to 0.01% of normal concrete.
- ✓ In failure pattern it is observed that the CC fails into two parts where as in ECC only crack is developed which reflects its ductile behavior.

6. REFERENCES

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